## **DESIGN CRITERIA NOTES**:

THE FOLLOWING CODES AND STANDARDS. INCLUDING ALL SPECIFICATIONS REFERENCED WITHIN, SHALL APPLY TO THE DESIGN. CONSTRUCTION, QUALITY CONTROL AND SAFETY OF ALL WORK PERFORMED ON THE PROJECT. USE THE LATEST EDITIONS UNLESS NOTED OTHERWISE.

INTERNATIONAL BUILDING CODE, 2018 EDITION ASCE 7-16 - MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES AISC 360-16 – SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS AISC 303-16 - CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES ACI 318-14 – BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE AND MASONRY TMS 402/602-16 – SPECIFICATION OF MASONRY STRUCTURES

DESIGN LOADS ARE AS LISTED BELOW.

# (UNIFORMLY DISTRIBUTED LIVE LOADS IN PSF)

MECHANICAL ROOMS 100 PSF OR EQUIP. WT.

SUPERIMPOSED DEAD LOADS:

WIND LOADS:

MECHANICAL, ELECTRICAL AND CEILING FINISHES WHERE SHOWN ON ARCHITECTURAL AS REQUIRED

BASIC WIND SPEED, V (ULT) = 115 MPH RISK CATEGORY = II WIND EXPOSURE = B APPLICABLE INTERNAL PRESSURE COEFFICIENT +/- 0.18

SEISMIC LOADS: RISK CATEGORY = II IMPORTANCE FACTOR = 1.0 MAPPED SPECTRAL RESPONSE ACCELERATIONS: Ss = 13.6%g

S1 = 4.3%a SPECTRAL RESPONSE COEFFICIENTS: SDS = 14.6%g SD1 = 6.9%g SITE CLASS = D SEISMIC DESIGN CATEGORY = B DESIGN BASE SHEAR = 28k SEISMIC RESPONSE COEFFICIENT, Cs = 0.048

ANALYSIS PROCEDURE = EQUIVALENT LATERAL FORCE

SNOW LOADS: GROUND SNOW LOAD Pg = 30 PSF FLAT ROOF SNOW LOAD = 21 PSF + DRIFT EXPOSURE FACTOR Ce = 1.0 IMPORTANCE FACTOR = 1.0 THERMAL FACTOR Ct = 1.0

RESPONSE MODIFICATION FACTOR, R = 3

## **FOUNDATION NOTES:**

FOUNDATION DESIGN AND SUBSURFACE INFORMATION IS BASED ON THE GEOTECHNICAL RECOMMENDATIONS PREPARED BY HILL-CARNES ENGINEERING ASSOCIATES, INC. DATED DECEMBER 5, 2022. THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND BE THOROUGHLY FAMILIAR WITH THE SITE AND SUBGRADE INFORMATION GIVEN THEREIN. ALL SITE PREPARATION AND EXCAVATION WORK IS TO BE PERFORMED IN ACCORDANCE WITH THE GEOTECHNICAL REPORT.

- SPREAD FOOTINGS ARE DESIGNED FOR THE ALLOWABLE NET SOIL BEARING PRESSURE OF 2000 PSF. SOIL BEARING CAPACITY SHALL BE VERIFIED BY A LICENSED GEOTECHNICAL ENGINEER PRIOR TO FOUNDATION PLACEMENT.
- FOOTINGS SHALL BE OVER-EXCAVATED TO A MINIMUM DEPTH OF 24 INCHES BELOW THE DESIGN BEARING ELEVATIONS AND BACKFILLED WITH LEAN CONCRETE (MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2.000 PSI). NO WIDENING OF THE OVER-EXCAVATION BELOW THE BOTTOM OF FOOTING IS REQUIRED. UPON COMPETITION OF THE OVER-EXCAVATION BELOW THE FOOTINGS AND PRIOR TO FOUNDATION CONSTRUCTION. THE EXPOSED SUBGRADE SOILS SHOULD BE COMPACTED WITH A WALK-BEHIND VIBRATORY ROLLER, AUTOMATIC TAMPER, OR OTHER SIMILAR EQUIPMENT AS APPROVED. FOUNDATIONS SHOULD BE CONSTRUCTED ON FIRM, DRY, NON-FROZEN SUBGRADE. AFTER COMPACTION EFFORTS ARE COMPLETED, FOOTING EXCAVATIONS SHOULD BE OBSERVED BY A GEOTECHNICAL ENGINEER OR EXPERIENCED SOILS TECHNICIAN PRIOR TO THE PLACEMENT OF REINFORCEMENT STEEL AND CONCRETE.
- PROVIDE CRACK CONTROL JOINTS IN SLABS-ON-GRADE AS INDICATED BY THE TYPICAL DETAILS. WHERE SOIL SUPPORTED SLAB CONSTRUCTION OR CONTROL JOINTS ARE NOT SHOWN ON PLAN, JOINTS SHALL OCCUR IN EACH DIRECTION AT EACH COLUMN LINE AND RE-ENTRANT CORNER. ADDITIONAL JOINTS SHALL OCCUR IN EACH DIRECTION AT A SPACING NOT TO EXCEED 36 TIMES THE SLAB THICKNESS. AVOID EXCEEDING AN ASPECT RATIO OF 1.5 TO 1 FOR AREAS BOUNDED BY JOINTS. CUT JOINTS 25% OF THE DEPTH OF THE SLAB.
- A VAPOR BARRIER PER THE ARCHITECTURAL SPECIFICATIONS SHALL BE INSTALLED DIRECTLY BENEATH THE SOIL SUPPORTED SLAB. TAPE AND LAP ALL JOINTS PER MANUFACTURER RECOMMENDATIONS.
- FOUNDATIONS SHALL BEAR ON UNDISTURBED VIRGIN SOIL AND/OR SUPERVISED COMPACTED FILL, FREE OF FROST AND ANY
- ALL FOUNDATIONS AND EXCAVATIONS SHALL BE PROTECTED FROM FROST EXPOSURE DURING AND AFTER CONSTRUCTION. BOTTOM OF ALL EXTERIOR FOOTINGS SHALL BE A MINIMUM OF 3'-0" BELOW EXTERIOR FINISHED GRADE, U.N.O.
- DO NOT ALLOW SURFACE WATER TO ACCUMULATE AND/OR POND IN EXCAVATIONS. TEMPORARY DEWATERING SYSTEM TO BE USED DURING CONSTRUCTION WILL BE DESIGNED AND INSTALLED BY THE CONTRACTOR IN ACCORDANCE WITH THE RECOMMENDATIONS GIVEN IN THE GEOTECHNICAL REPORT AND THE REQUIREMENTS OF THE GOVERNING BUILDING CODE.
- DO NOT BACKFILL AGAINST WALLS UNTIL SUPPORTING SLABS ARE IN PLACE AND HAVE ATTAINED REQUIRED STRENGTH. WHERE PIPES OR UTILITIES PASS THROUGH FOOTING, DROP FOOTING SO UTILITIES PASS OVER THE TOP OF THE FOOTING OR
- DROP UTILITIES TO PASS BELOW FOUNDATION 6" (MIN.) PROVIDE SLEEVES AS NECESSARY.
- 11. REFER TO THE ABOVE MENTIONED GEOTECHNICAL REPORT FOR ADDITIONAL SUBGRADE PREPARATION AND OTHER REQUIREMENTS.

# **CONCRETE REINFORCEMENT NOTES:**

- CONCRETE REINFORCING BARS SHALL BE NEW DOMESTIC DEFORMED BILLET STEEL CONFORMING TO ASTM A615 GRADE 60, EXCEPT AS NOTED. [COLUMN TIES] AND FIELD BENT #3 DOWELS MAY BE ASTM A615, GRADE 40. REINFORCEMENT REQUIRED TO BE WELDED SHALL CONFORM TO ASTM A706, U.N.O.
- WELDED WIRE REINFORCING SHALL CONFORM TO ASTM A185. THE FOLLOWINGWELDED WIRE REINFORCING SHALL BE USED FOR AREAS SPECIFIED BELOW, UNLESS NOTED OTHERWISE ON THE DRAWINGS:

5 INCH SLAB-ON-GRADE 6 X 6 - W2.9 X W2.9 6 INCH SLAB-ON-GRADE 6 X 6 - W2.9 X W2.9

FOR ADDITIONAL WWF, IF ANY, SEE FLOOR FRAMING PLANS.

- REINFORCING BARS MAY BE SPLICED ONLY AS SHOWN ON THE DRAWINGS EXCEPT THAT REINFORCING DESIGNATED AS "CONTINUOUS" SHALL HAVE A CLASS "B" LAP SPLICE (ACI 318). LAP SPLICES OF CONTINUOUS REINFORCING SHALL BE MADE OVER SUPPORTS FOR BOTTOM BARS AND FOR INTERMEDIATE BARS AND AT MIDSPAN FOR TOP BARS. AT EXTERIOR SUPPORTS, TOP AND BOTTOM BARS SHALL BE HOOKED AND INTERMEDIATE BARS SHALL EXTEND TO WITHIN 2" OF EXTERIOR FACE.
- HORIZONTAL WALL REINFORCEMENT SHALL BE CONTINUOUS AND SHALL HAVE 90 DEGREE BENDS AND EXTENSIONS AT CORNERS AND INTERSECTIONS AS SHOWN ON TYPICAL WALL CORNER BAR PLACING DETAILS.
- 5. LAPS IN WELDED WIRE FABRIC SHALL BE TWO MESH AT SPLICES.
- PROVIDE STANDARD BAR CHAIRS WITH PROTECTIVE TIPS AND SPACERS SPACED AS REQUIRED TO PROVIDE SPECIFIED CONCRETE PROTECTION FOR REINFORCEMENT BUT NOT TO EXCEED 3'-0" ON CENTER FOR SLABS, BEAMS, AND GRADE BEAMS. PLACE BAR CHAIRS LONGITUDINALLY IN BEAMS DIRECTLY BELOW THE STIRRUPS.
- ALL REINFORCING SHALL BE SECURELY WIRED TOGETHER IN THE FORMS PRIOR TO PLACING CONCRETE.
- THE CONTRACTOR SHALL SUBMIT AND RECEIVE APPROVAL OF REINFORCING STEEL SHOP DRAWINGS PRIOR TO FABRICATION OR SHIPMENT OF MATERIAL. SHOP DRAWINGS SHALL CONTAIN INFORMATION FOR DETAILING, SPLICING, LAPPING, BENDING, SHAPES. QUANTITIES AND DIMENSIONS OF ALL BAR REINFORCEMENT INCLUDING SUPPORTING AND SPACING DEVICES.

#### CAST-IN-PLACE CONCRETE NOTES:

- 1. ALL CONCRETE WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE LATEST EDITIONS OF THE ACI BUILDING CODE (ACI 318), ACI DETAILING MANUAL (ACI 315), AND SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301).
- 2. CONCRETE, UNLESS OTHERWISE NOTED ON THE PLANS, SHALL BE NORMAL WEIGHT CONCRETE WITH THE FOLLOWING 28 DAY COMPRESSIVE STRENGTH (PSI). ALL CONC, U.N.O. = 3,000 N.W.C.
- WALLS = 4,000 N.W.C.
- SLAB-ON-GRADE = 3,500 N.W.C.
- EXTERIOR SLAB-ON-GRADE = 4,500 N.W.C.
- \* N.W.C.-DENOTES NORMAL WEIGHT CONC. WITH A MAX. DRY DENSITY = 150 PCF 3. CONCRETE PROTECTION FOR REINFORCEMENT SHALL BE AS NOTED BELOW. SEE ACI 318 FOR CONDITIONS NOT NOTED.
- REINF. STEEL IN CONCRETE CAST AGAINST SOIL = 3"
- REINF. STEEL IN CONCRETE EXPOSED TO SOIL OR WEATHER:
- #5 BARS AND SMALLER = 1.5" #6 BARS AND LARGER = 2"
- CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND: SLAB AND WALL REINF. = 0.75"
- 4. ALL CONCRETE EXPOSED TO WEATHER SHALL BE AIR ENTRAINED TO 5% +/- 1%.
- WORKABILITY SHALL NOT BE ACHIEVED THROUGH ADDITION OF WATER. WATER REDUCING ADMIXTURES (PLASTICIZERS) SHALL BE USED TO INCREASE WORKABILITY. SEE SPECIFICATIONS FOR CONCRETE SLUMP REQUIREMENTS.
- HORIZONTAL CONSTRUCTION JOINTS SHALL BE PERMITTED ONLY WHERE SHOWN ON THE STRUCTURAL DRAWINGS.
- CONTROL JOINTS FOR SLABS ON GRADE SHALL BE SAW CUT PER THE TYPICAL DETAILS ON THE STRUCTURAL DRAWINGS. DIAMOND LEAVE OUTS SHALL BE PROVIDED AT ALL COLUMNS.
- CONTRACTOR SHALL SUBMIT PLAN SHOWING POUR SEQUENCE, INCLUDING TYPE AND LOCATION OF PROPOSED JOINTS IN SLABS AND WALLS FOR APPROVAL
- 9. ALL CONCRETE PLACED IN COLD WEATHER SHALL CONFORM TO ACI 306-COLD WEATHER CONCRETING. ALL CONCRETE PLACED IN HOT WEATHER SHALL CONFORM TO ACI 305-HOT WEATHER CONCRETING.
- 10. CONTRACTOR SHALL COORDINATE THE LOCATION OF ALL SLEEVES, INSERTS, ANCHOR BOLTS AND OTHER EMBEDDED ITEMS AS
- REQUIRED BY OTHER TRADES. TOP SURFACES OF CONCRETE BEAMS TO SUPPORT PRECAST CONCRETE MEMBERS NOT PLACED IN THE FORMS SHALL RECEIVE A
- 12. ALL CONCRETE POURS SHALL BE TERMINATED BY FORMS. FOOTINGS MAY BE UNFORMED PROVIDED THE TRENCH IS EXCAVATED AN ADDITIONAL 3" ON ALL SIDES, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- 13. ALL COLUMN FOOTINGS SHALL BE CENTERED UNDER COLUMN CENTERLINES, U.N.O.

TROWELLED SMOOTH FINISH AND SHALL BE STRAIGHT, TRUE AND LEVEL.

- 14. GROUT ALL LEVELING AND BEARING PLATES WITH AN APPROVED NON-SHRINK GROUT.
- 15. CHAMFER CORNERS OF ALL EXPOSED CONCRETE AS DETAILED BY THE ARCHITECTURAL DRAWINGS.
- 16. A THIRD PARTY TESTING LABORATORY SHALL BE EMPLOYED BY THE OWNER FOR EVALUATION AND QUALITY CONTROL OF CONCRETE PLACED. FREQUENCY OF CONCRETE TESTING SHALL MEET THE REQUIREMENTS OF ACI 318 AT A MINIMUM UNLESS OTHERWISE REQUIRED BY THE LOCAL BUILDING CODE.

#### CONCRETE MASONRY UNITS NOTES:

- ALL CONCRETE MASONRY WALLS AND COLUMNS SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE "BUILDING CODE REQUIREMENT FOR MASONRY STRUCTURES" (ACI 530/ASCE 5/TSM 402) AND "SPECIFICATIONS FOR MASONRY STRUCTURES" (ACI 530.1/ASCE 6/TSM 602)/ LATEST EDITIONS.
- ALL MASONRY UNITS SHALL BE ASTM C90, TYPE 1 MEDIUM WEIGHT WITH MINIMUM COMPRESSIVE STRENGTH OF 1,900 PSI AT 28 DAYS ON THE NET AREA OF INDIVIDUAL UNITS. ALL CMU SHALL BE ERECTED IN RUNNING BOND, ON FULL MORTAR BEDS, UNLESS OTHERWISE NOTED.
- CMU MORTAR SHALL BE PORTLAND CEMENT-LIME CONFORMING TO ASTM C270. USE TYPE "M" FOR MASONRY BELOW GRADE AND USE TYPE "M" OR "S" FOR ALL INTERIOR AND EXTERIOR WALLS ABOVE GRADE.
- 4. CMU GROUT, POURED OR PUMPED, SHALL MEET ASTM C476, AND HAVE MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3000 PSI.
- PROVIDE HOT DIPPED GALVANIZED LADDER TYPE HORIZONTAL JOINT REINFORCEMENT (9 GA.) AT 16" ON CENTER VERTICAL IN ALL MASONRY WALLS WITH PREFABRICATED "T" AND CORNER PIECES. LAP SPLICED 6" MIN. PROVIDE AN ADDITIONAL ROW ABOVE AND BELOW OPENINGS AND EXTEND 2'-0" BEYOND JAMBS. STOP HORIZONTAL REINFORCING EACH SIDE OF CONTROL AND EXPANSION JOINTS. HORIZONTAL JOINT REINFORCING SHALL MEET ASTM A82.
- 6. SEE STRUCTURAL DRAWINGS FOR REINFORCING OF LOADBEARING CMU WALLS. ALL CMU WALLS 8" OR WIDER SHALL HAVE THE FOLLOWING MINIMUM REINFORCING UNLESS NOTED OTHERWISE: INTERIOR WALLS -#4 AT 72" O.C.; EXTERIOR WALLS - #5 AT 48" O.C.
- IN ADDITION TO REINFORCING STEEL NOTED ON PLANS, SCHEDULES, AND SECTIONS, PROVIDE VERTICAL BARS (#4 MIN, U.N.O.) WITHIN 8" OF EACH SIDE OF WALL CONTROL JOINTS, WITHIN 8" OF THE ENDS OF WALLS, WITHIN 16" OF EACH SIDE OF OPENINGS, AND AT ALL CORNERS. DOWELS TO MATCH VERTICAL REINF. WHERE CMU BEARS ON EXISTING SLAB, PROVIDE DOWEL TO EXISTING SLAB WITH ADHESIVE ANCHOR SYSTEM PER TYP. DETAILS (3" MIN. EMBED.).
- SEE ARCHITECTURAL DRAWINGS FOR LOCATION OF MASONRY CONTROL JOINTS. IF NOT INDICATED ON THE DRAWINGS, CONCRETE MASONRY SHALL HAVE VERTICAL CONTROL JOINTS WITH A MAXIMUM SPACING OF 24'-0" O.C. AT EXTERIOR WALLS BUT NOT LESS THAN 8" FROM BEARING PLATES.
- CONTROL JOINTS SHALL ALSO BE LOCATED AT ABRUPT CHANGES IN WALL HEIGHT, CHANGES IN WALL THICKNESS, WALL CORNERS, INTERSECTIONS OF WALLS WITH COLUMNS, PIERS AND PILASTERS, AND NO CLOSER THAN 2'-0" TO EDGE OF ANY WALL OPENING. REINFORCING BARS IN BOND BEAMS AT ROOF AND FLOOR ELEVATIONS SHALL BE CONTINUOUS ACROSS CONTROL JOINTS. WHERE DISCREPANCIES EXIST BETWEEN THE ARCH'L DRAWINGS AND CONTROL JOINT SPACING REQUIREMENTS, THE CONTRACTOR SHALL NOTIFY THE ARCHITECT AND ENGINEER TO RESOLVE THE DISCREPANCY.
- 10. SPECIAL INSPECTION IS [NOT] REQUIRED FOR CMU LOADBEARING WALLS.
- 11. ALL CORES THAT CONTAIN VERTICAL REINFORCING SHALL BE FILLED SOLID WITH GROUT. ALL CMU PIERS SHALL BE COMPLETELY FILLED WITH GROUT. FILLING CORES WITH MORTAR AS WORK PROGRESSES IS NOT ACCEPTABLE.
- 12. ALL CMU THAT HAS ONE OR MORE FACES BELOW GRADE SHALL BE GROUTED SOLID.
- THE MINIMUM SPLICE LENGTH FOR ALL VERTICAL BARS IS 48 BAR DIAMETERS UNLESS OTHERWISE NOTED. LAP SPLICES SHALL OCCUR DIRECTLY ABOVE FOOTINGS AND SLABS. NO SPLICES ARE ALLOWED AT MID-HEIGHT OF WALL.
- REINFORCING BAR POSITIONERS SHALL BE USED TO HOLD BARS IN THE PROPER LOCATION. POSITIONERS SHALL BE PLACED AT A
- MAXIMUM VERTICAL SPACING OF 48" O.C. 15. ALL UNITS SHALL BE CONTINUOUSLY PROTECTED FROM EXPOSURE TO RAIN OR OTHER SOURCES OF WATER FROM TIME OF

CASTING TO FINAL PLACEMENT OF THE WALL. ALL CONCRETE MASONRY UNITS SHALL BE DRY, FEEL FROM SOIL, ICE, AND FROST

- WHEN LAID IN THE WALL. 16. LAY NO MASONRY WHEN THE TEMPERATURE OF OUTSIDE AIR IS BELOW 40 DEGREES FAHRENHEIT, UNLESS SUITABLE MEANS ARE PROVIDED TO HEAT MATERIALS, PROTECT WORK FROM COLD AND FROST, AND ENSURE THAT THE MORTAR WILL HARDEN WITHOUT FREEZING. COMPLY WITH THE COLD WEATHER REQUIREMENTS CONTAINED IN ACI 530.1.
- 17. ALL BEAMS SUPPORTING MASONRY, INCLUDING STEEL, PRECAST, AND MASONRY LINTELS ARE TO BEAR ON 8" MIN. AND ON 3 COURSES GROUTED SOLID, MINIMUM.
- UNLESS NOTED OTHERWISE, PROVIDE ANCHORAGE OF MASONRY WALLS TO THE STRUCTURE IN THE FOLLOWING MANNER: AT STEEL BEAMS - ADJUSTABLE MASONRY ANCHORS AT 16" AT CONCRETE ELEMENTS – DOVETAIL ANCHORS AT 16"
- 19. SPACE MASONRY VENEER ANCHORS AT A MAXIMUM OF 32" HORIZONTALLY AND 18" VERTICALLY. PROVIDE ADDITIONAL ANCHORS AROUND OPENINGS LARGER THAN 16" IN EITHER DIMENSION. SPACE ANCHORS AROUND PERIMETER OF OPENING AT A MAXIMUM OF 32" O.C. PLACE ANCHORS WITHIN 12" OF OPENINGS.
- ALL LOOSE ANGLE LINTELS SHALL HAVE A MINIMUM BEARING OF 6" ON MASONRY. WHERE LOOSE ANGLES ARE EXPOSED TO WEATHER, ANGLES SHALL BE GALVANIZED. LOOSE LINTELS SHALL BE PROVIDED AT OPENINGS 12" WIDE OR LARGER. UNLESS NOTED OTHERWISE, WIDTH OF LINTEL SHALL BE 1/2" LESS THAN WIDTH OF MASONRY BEING SUPPORTED (5/16" THICKNESS).

# 21. INSTALL FLASHING AS REQUIRED BY THE ARCH'L DRAWINGS.

- REMOVAL AS DESCRIBED HEREIN SHALL BE ACCOMPLISHED WITHOUT STORING ON THE FLOOR EXCESSIVE QUANTITIES OF ANY MATERIALS, RUBBISH, DIRT, DEBRIS, OR WASTE OF ANY SORT RESULTING FROM THE REMOVAL OPERATIONS ON THE FLOOR. 2. ALL DEBRIS SHALL BE REMOVED FROM THE CONSTRUCTION SITE DAILY.
- THE CONTRACTOR SHALL TAKE PRECAUTIONS TO MAINTAIN FREE PROTECTED ACCESS OF ALL TENANTS, SERVICE PERSONNEL AND THE PUBLIC THROUGH THE AREAS INVOLVED.
- 4. THE CONTRACTOR SHALL REMOVE ALL PIPE SLEEVES PROJECTING THROUGH SLAB; PATCH ALL PENETRATIONS, HOLES, ETC.
- 5. ALL PIPES AND CONDUITS IN WALLS THAT ARE TO BE DEMOLISHED ARE TO BE REMOVED AND/ OR RELOCATED AS REQUIRED.
- CONTRACTOR SHALL REVIEW WITH ARCHITECTS/ ENGINEER ANY AND ALL ITEMS OF DEMOLITION NOT IMPLIED OR SPECIFIED ON DRAWINGS OR SPECIFICATIONS AND TO INCLUDE SUCH COSTS IN BID UNLESS OTHERWISE ADVISED.
- 7. PROVIDE ALL LABOR, MATERIAL, EQUIPMENT AND SERVICES AND PERFORM ALL OPERATIONS REQUIRED FOR COMPLETE INTERIOR DEMOLITION AND RELATED WORK AS DESCRIBED AND SPECIFIED HEREIN, AND AS MAY BE REASONABLY IMPLIED AS NECESSARY COMPLETE WORK IN ALL RESPECTS.
- 8. JOBSITE INSPECTION MUST BE CONDUCTED TO EXAMINE EXISTING CONDITIONS, TO DETERMINE NATURE AND SCOPE OF WORK OR ANY DIFFICULTIES THAT MIGHT ARISE AT TIME OF WORK. IN ADDITION, EXAMINE ALL WORK THAT IS INTENDED TO REMAIN AS PART OF THE COMPLETED PROJECT AND REPORT ALL UNSATISFACTORY CONDITIONS TO ARCHITECT/ ENGINEER PRIOR TO COMMENCEMENT OF WORK. EXERCISE EXTREME CARE DURING DEMOLITION SO AS NOT TO DAMAGE CONSTRUCTION AND OTHER STRUCTURES THAT ARE INTENDED TO REMAIN. ANYTHING DAMAGED AT WORK IS TO BE REPAIRED AND/ OR REPLACED TO MATCH EXISTING CONSTRUCTION AT CONTRACTORS EXPENSE.
- 9. REFER TO DRAWINGS FOR EXISTING ITEMS/ SYSTEMS TO REMAIN.
- CONTRACTOR TO PROVIDE DUST BARRIER FOR PROTECTION OF EXISTING AREAS TO REMAIN AS REQUIRED.
- 11. WHEN DEMOLITION TAKES PLACE, SHOULD ANY WORK AFFECT THE INTEGRITY OF THE STRUCTURE, WORK MUST STOP IMMEDIATELY, AND ARCHITECT/ ENGINEER NOTIFIED. UNDER NO CIRCUMSTANCES SHALL REINFORCING OF ANY KIND BE DAMAGED,
- 12. THE GENERAL CONTRACTOR SHALL PROVIDE SUFFICIENT FRAMING FOR ALL WALL OPENINGS FOR DUCTWORK, RETURN AIR OPENINGS, ACCESS PANELS AND GRILLE OPENINGS ABOVE AND BELOW HUNG CEILINGS. THESE ARE TO BE COORDINATED WITH H.V.A.C. ENGINEERING DRAWINGS AND THE GENERAL CONTRACTOR'S SHOP DRAWINGS AND THE GENERAL CONTRACTOR'S MECHANICAL CONTRACTOR'S SHOP DRAWINGS. ALL SPACES SHALL BE PROPERLY SEALED FOR SOUNDPROOFING AND VIBRATION.
- 13. PRIOR TO DEMOLITION OF LOAD BEARING MEMBERS, SUPPORTED MEMBERS SHALL BE SHORED.

#### STEEL NOTES:

- A. STRUCTURAL STEEL
- STRUCTURAL STEEL CONSTRUCTION HAS BEEN DESIGNED IN ACCORDANCE WITH A.I.S.C. 360-05, "STEEL CONSTRUCTION
- STRUCTURAL STEEL SHAPES, PLATES, ETC., SHALL CONFORM TO THE FOLLOWING ASTM DESIGNATIONS, U.N.O.

FLOOR/ROOF BEAMS AND GIRDERS ASTM A992-50 COLUMNS, WEB DOUBLER PLATES ASTM A992-50 CHANNELS, TEES, ANGLES, BARS, PLATES, ASTM A36 STEEL TUBING (HSS SECTIONS) ASTM A500-GR. B (Fy = 46 KSI) STEEL PIPE **ASTM A501 OR A53** TYPE "E" OR "S" ANCHOR BOLTS ASTM F1554 GR. 36 ASTM F1554 GR. 55

- CONNECTION BOLTS SHALL CONFORM TO ASTM A325. USE BEARING TYPE BOLTS WITH THREAD ALLOWED ACROSS THE SHEAR PLANE (TYPE N) AT TYPICAL BEAM SHEAR CONNECTIONS, U.N.O. USE TYPE "SC" BOLTS WITH EITHER DIRECT TENSION INDICATOR OR LOAD INDICATOR WASHERS AT ALL BOLTED SLIP CRITICAL CONNECTIONS.
- 4. A LISTING OF CONNECTIONS CONSIDERED "SLIP CRITICAL" IS AS FOLLOWS: A. BOLTED CONNECTIONS OF TENSION MEMBERS.
- BOLTS USED IN MOMENT CONNECTIONS. BOLTED SPLICES OF TRUSS TOP AND BOTTOM CHORDS.
- STEEL BEAM CONNECTIONS NOT DETAILED ON THE DRAWINGS SHALL BE DESIGNED BY THE STRUCTURAL STEEL FABRICATOR. BEAM CONNECTIONS SHALL DEVELOP THE END REACTIONS GIVEN ON THE DRAWINGS. WHERE END REACTIONS ARE NOT SPECIFIED. THE BEAM CONNECTION SHALL DEVELOP 50% OF THE BEAMS WEB ALLOWABLE SHEAR CAPACITY. A MINIMUM CONNECTION CAPACITY OF 12 KIPS SHALL BE PROVIDED FOR ALL BEAMS. UNLESS NOTED OTHERWISE BY SPECIFIED REACTION. THE STRUCTURAL STEEL FABRICATOR SHALL PROVIDE CERTIFICATION BY A PROFESSIONAL ENGINEER, REGISTERED IN THE STATE OF THE PROJECT. THAT THE CONNECTION DESIGN IS IN ACCORDANCE WITH ALL APPLICABLE CODES AND SPECIFICATIONS.
- 6. FOR ALL HIGH STRENGTH BOLTS, HARDENED WASHERS SHALL BE PROVIDED.
- 7. GALVANIZING OF STEEL MEMBERS SHALL CONFORM TO ASTM A123. GALVANIZE ALL STEEL PERMANENTLY EXPOSED TO WEATHER. ALL STRUCTURAL STEEL SHALL BE SHOP PAINTED WITH A RUST INHIBITIVE PRIMER. DO NOT PRIME STEEL WHICH SHALL HAVE SPRAY-ON FIREPROOFING APPLIED. STEEL WHICH IS TO BE FIREPROOFED IS INDICATED ON THE ARCHITECTURAL DRAWINGS. ALL EXPOSED STEEL AND LINTELS IN EXTERIOR WALLS SHALL BE HOT-DIP GALVANIZED.
- HEADED STUDS AND DEFORMED BAR ANCHORS USED IN FABRICATION OF EMBEDDED ASSEMBLIES SHALL BE WELDED TO THOSE ASSEMBLIES USING A FULL FUSION PROCESS.
- 10. STEEL BEAMS SHALL BE ERECTED WITH NATURAL CAMBER UP.
- 11. ANCHOR BOLTS HAVE NOT BEEN DESIGNED FOR ANY SPECIFIC ERECTION FORCES. THE ERECTOR IS RESPONSIBLE FOR ANY AND ALL GUYING AND BRACING REQUIRED TO ERECT THE BUILDING.
- 12. COMPOSITE BEAMS USING CONCRETE SLAB AS COMPRESSION FLANGE ARE DESIGNED FOR UNSHORED CONSTRUCTION. THE CONTRACTOR SHOULD ANTICIPATE UP TO 5/8" DEFLECTION UNDER WET WEIGHT OF CONCRETE FOR BEAMS WHICH HAVE NO CAMBER SHOWN ON THE STRUCTURAL DRAWINGS.
- 13. OPEN WEB STEEL JOISTS AND BRIDGING SHALL CONFORM TO THE STANDARDS OF THE STEEL JOIST INSTITUTE. BRIDGING SHALL BE WELDED TO STEEL BEAMS AND ANCHORED TO MASONRY OR CONCRETE WALLS AT THE ENDS, U.N.O. JOISTS SHALL BE WELDED TO STEEL SUPPORTS PER SJI SPECIFICATIONS. JOISTS SUPPORTED BY A COLUMN SHALL BE WELDED TO THE COLUMN WITH 1/8"X2" LONG FILLET WELDS EACH SIDE UNLESS NOTED OTHERWISE. JOISTS SUPPORTED ON STEEL BEAMS, GIRDERS, OR BEARING PLATES SHALL HAVE THE JOIST SEAT DESIGNED TO TRANSFER THE LOAD TO THE SUPPORT CENTERLINE UNLESS NOTED
- 14. THE RESPONSIBILITY FOR ANY TEMPORARY SHORING OR BRACING DURING THE CONSTRUCTION PHASE BEFORE COMPLETION OF CONNECTION AND POURING OF FLOOR SLAB IS ADDRESSED IN THE SPECIFICATIONS AND IS THE RESPONSIBILITY OF THE
- 15. IF NOT SHOWN ON DRAWINGS, SUPPORT OF METAL DECK AROUND COLUMN CLOSURE, SCREED PLATES AROUND THE OPENINGS AND EDGE SLAB SHALL BE PROVIDED BY THE CONTRACTOR.
- 16. DURING CONSTRUCTION, THE ERECTED STRUCTURAL STEEL SHALL NOT PROCEED HIGHER THAN THE CONCRETE CORE CONSTRUCTION. THE CONTRACTOR SHALL MAKE SAFE PROVISIONS FOR STABILIZING THE STEEL STRUCTURE BOTH HORIZONTALLY AND VERTICALLY. THE STABILITY OF THE FRAME DURING ERECTION IS THE CONTRACTOR'S RESPONSIBILITY.
- STRUCTURAL STEEL MEMBERS SHALL NOT BE SPLICED OR HAVE PENETRATIONS UNLESS INDICATED ON THE STRUCTURAL CONTRACT DOCUMENTS OR AS REVIEWED BY THE STRUCTURAL ENGINEER.
- WELDED CONSTRUCTION SHALL CONFORM TO THE AMERICAN WELDING SOCIETY "STRUCTURAL WELDING CODE" D1.1; AWS D1.3-
- SHEET STEEL; AND AWS D1.4 "REINFORCING STEEL WELDING CODE". ELECTRODES FOR FIELD AND SHOP WELDS OF STRUCTURAL STEEL SHALL BE E70XX, U.N.O.
- 3. ELECTRODES FOR WELDING OF REINFORCING STEEL SHALL BE E80XX.
- 4. ELECTRODES FOR WELDING OF SHEET STEEL SHALL CONFORM TO AWS D1.3.
- WHEN WELDS ARE NOT CALLED-OUT ON DRAWINGS, THEY ARE MINIMUM SIZE CONTINUOUS FILLET WELDS IN ACCORDANCE WITH
- AWS D1.1. FILLET WELDS NOT SPECIFIED AS TO LENGTH SHALL BE CONTINUOUS.
- UNLESS NOTED OTHERWISE ON THE DRAWINGS, ALL GROOVE WELDS SHALL BE FULL PENETRATION. ONLY LOW HYDROGEN ELECTRODES SHALL BE USED ON REINFORCING STEEL AND ASTM A992 STEEL.
- PROVIDE FILLET WELDS AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS.

# SUBMITTAL AND SHOP DRAWING REQUIREMENT NOTES:

- THE CONTRACTOR SHALL SUBMIT FOR REVIEW BY THE ARCHITECT AND THE ENGINEER ALL INFORMATION REQUIRED BY THE
- CONTRACT DOCUMENTS INCLUDING THE SPECIFICATIONS. SHOP DRAWINGS SHALL BE PREPARED, SUBMITTED AND REVIEWED PRIOR TO PROCEEDING WITH FABRICATION AND/ OR
- REPRODUCTION OF ANY PORTION OF THE STRUCTURAL CONTRACT DRAWINGS FOR RE-SUBMITTAL AS SHOP DRAWINGS IS

INSTALLATION OF THE ASSOCIATED WORK. REVIEW PERIOD SHALL BE A MINIMUM OF TWO (2) WEEKS.

PROHIBITED. SHOP DRAWINGS PRODUCED IN SUCH A MANNER WILL BE REJECTED AND RETURNED. 4. SHOP DRAWINGS SHALL BE SUBMITTED ELECTRONICALLY.

THE ACTUAL EQUIPMENT PURCHASED.

- SHOP DRAWINGS SUBMITTED FOR REVIEW SHALL BEAR THE CONTRACTOR'S STAMP OF APPROVAL WHICH SHALL CONSTITUTE CERTIFICATION THAT THE CONTRACTOR HAS VERIFIED ALL CONSTRUCTION CRITERIA, DIMENSIONS, MATERIALS, AND SIMILAR DATA HAS CHECKED EACH DRAWING FOR COMPLETENESS, COORDINATION, AND COMPLIANCE WITH THE CONTRACT DOCUMENTS. THE CONTRACTOR'S REVIEW INCLUDES BUT IS NOT LIMITED TO VERIFICATION AND COORDINATION OF ACTUAL FIELD CONDITIONS, INCLUDING DIMENSIONS AND ELEVATIONS, AS WELL AS ACTUAL DIMENSIONS FOR SUPPORTS, ANCHORAGES, AND OPENINGS FOR
- THE SHOP DRAWINGS SHALL INCLUDE DIMENSIONED FLOOR AND FLOOR EDGES, OPENINGS AND SLEEVES AT ALL FLOORS

### REQUIRED FOR ALL TRADES. <u>Metal Deck Notes:</u>

COMPOSITE METAL DECK SHALL BE GALVANIZED AND SHALL BE PLACED WITH CONTINUOUS SPANS OF THREE OR MORE WHERE POSSIBLE. IN NO CASE SHALL UNSHORED METAL DECK SPANS EXCEED THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS OR DEFLECTION CRITERIA OF SPAN DIVIDED BY 240. METAL DECK SHALL PROVIDE THE FOLLOWING MINIMUM PROPERTIES:

TYPICAL COMPOSITE METAL DECK 2"--20 GA. (50 KSI) IP = 0.409 IN4/FT SP = 0.341 IN3/FT

IN = 0.406 IN4/FT.

SN = .346 IN3/FT.

- FLOOR DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL FLOOR DECK SHALL BE CONTINUOUS OVER A MINIMUM
- OF THREE SPANS. DECK UNITS SHALL BE LAPPED ONLY OVER SUPPORTS. 3. COMPOSITE SHEAR STUDS TO BE WELDED THROUGH STEEL DECK. STUD WORK TO BE IN ACCORDANCE WITH THE REQUIREMENTS
- OF THE NELSON STUD WELDING. SHEAR STUDS MUST COMPLY WITH ASCI A 108 (FY=50 KSI). 4. ROOF DECK OVER STEEL BEAMS SHALL BE DOVETAIL METAL ROOF DECK WITH THE FOLLOWING MINIMUM PROPERTIES:

2.0D--18 GA. (40 KSI) IP = 0.626 IN4/FT. SP = 0.463 IN3/FT

IN= 0.612 IN4/FT

INSULATING CONCRETE SHALL BE VENTED.

 $SN = 0.576 IN3/F^{-1}$ MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE. ROOF DECK MUST COMPLY WITH STEEL DECK INSTITUTE STANDARDS. ALL ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS. ROOF DECK WITH LIGHTWEIGHT

WELD DECK TO SUPPORTING MEMBERS ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. DECK AND WELDS SHALL HAVE A

- MINIMUM DESIGN DIAPHRAGM SHEAR CAPACITY OF 300 PLF. IN AREAS OF WARPED ROOF DECK, SELF DRILLING SCREWS ARE TO BE USED ON CONNECTIONS OF STEEL ROOF DECK TO STRUCTURAL STEEL SUPPORTS. SCREW SIZES TO COMPLY WITH MANUFACTURER'S AND FACTORY MUTUAL REQUIREMENTS.
- ATTACH DECK TO ALL SUPPORTING ROOF JOISTS. LOADS EXCEEDING 50 LBS SHALL NOT BE PERMITTED TO BE HUNG FROM ANY METAL DECKING. ALL HANGERS FOR PIPING. DUCTWORK, CABLE TRAYS, ETC. SHALL BE HUNG DIRECTLY FROM STRUCTURAL STEEL MEMBERS AND/OR ANCHORS EMBEDDED



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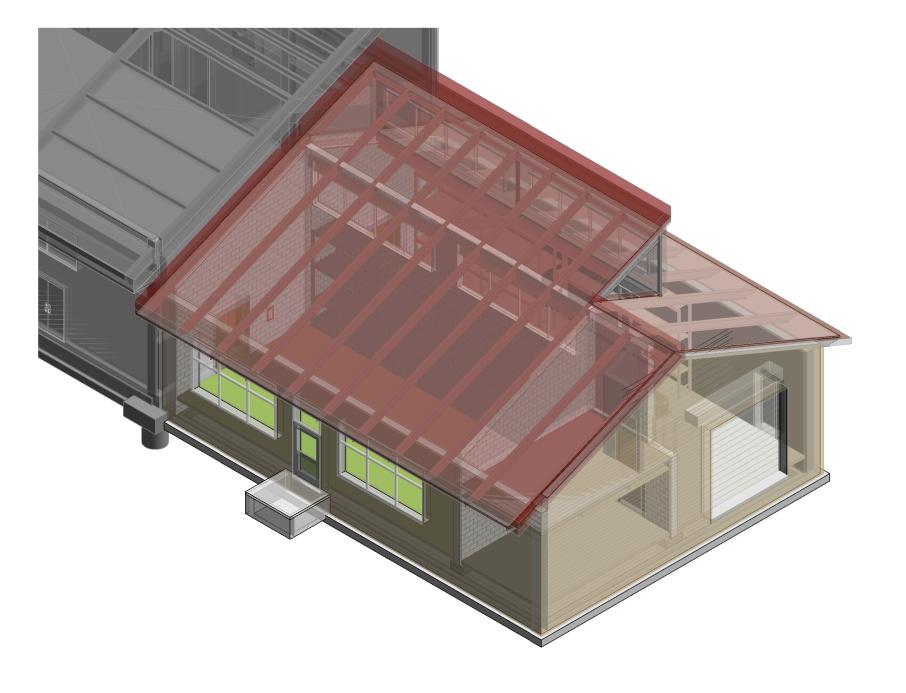
Consultants: STRUCTURAL: WZG Structural Consulting Engineers, Inc. 1137 Gravel Pike

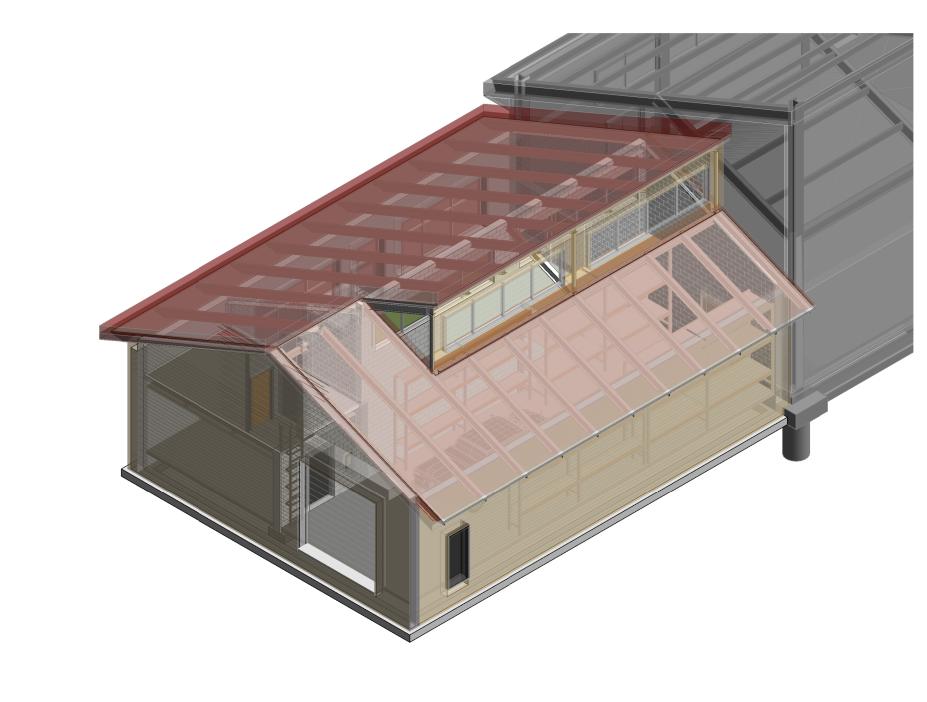
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ISSUE FOR PERMIT | 12/16/22

Date No.

PA GAME COMMISSION

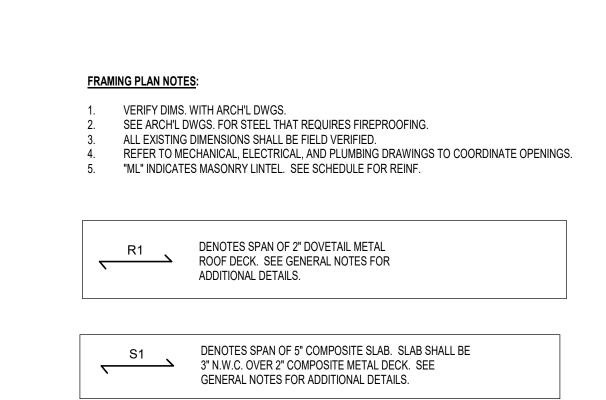
**ISSUE LOG** 

Distribution

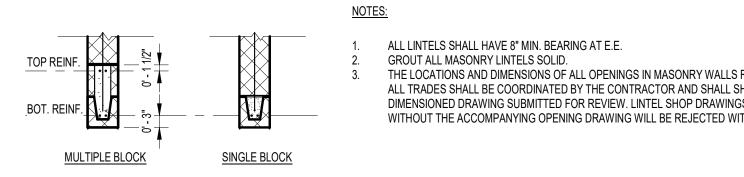
TRAINING WING ADDITION

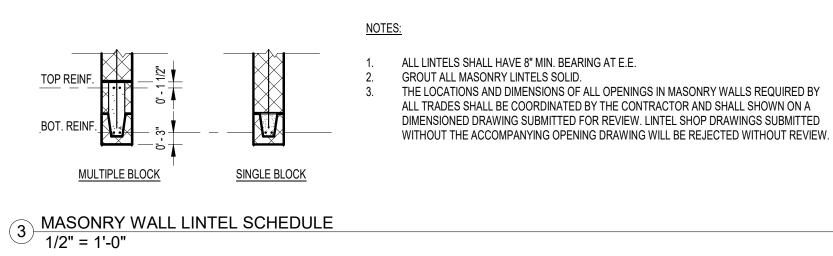
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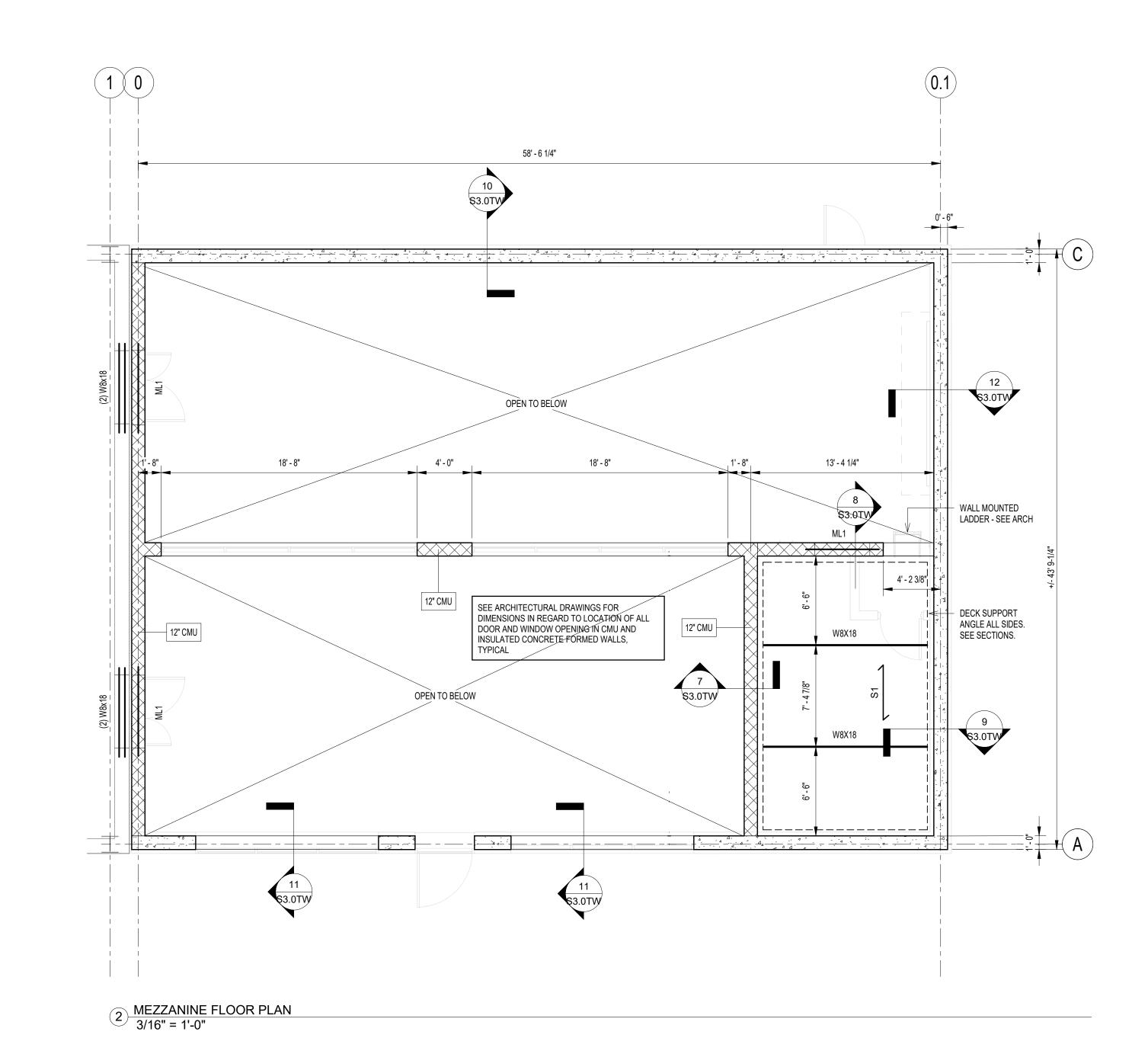


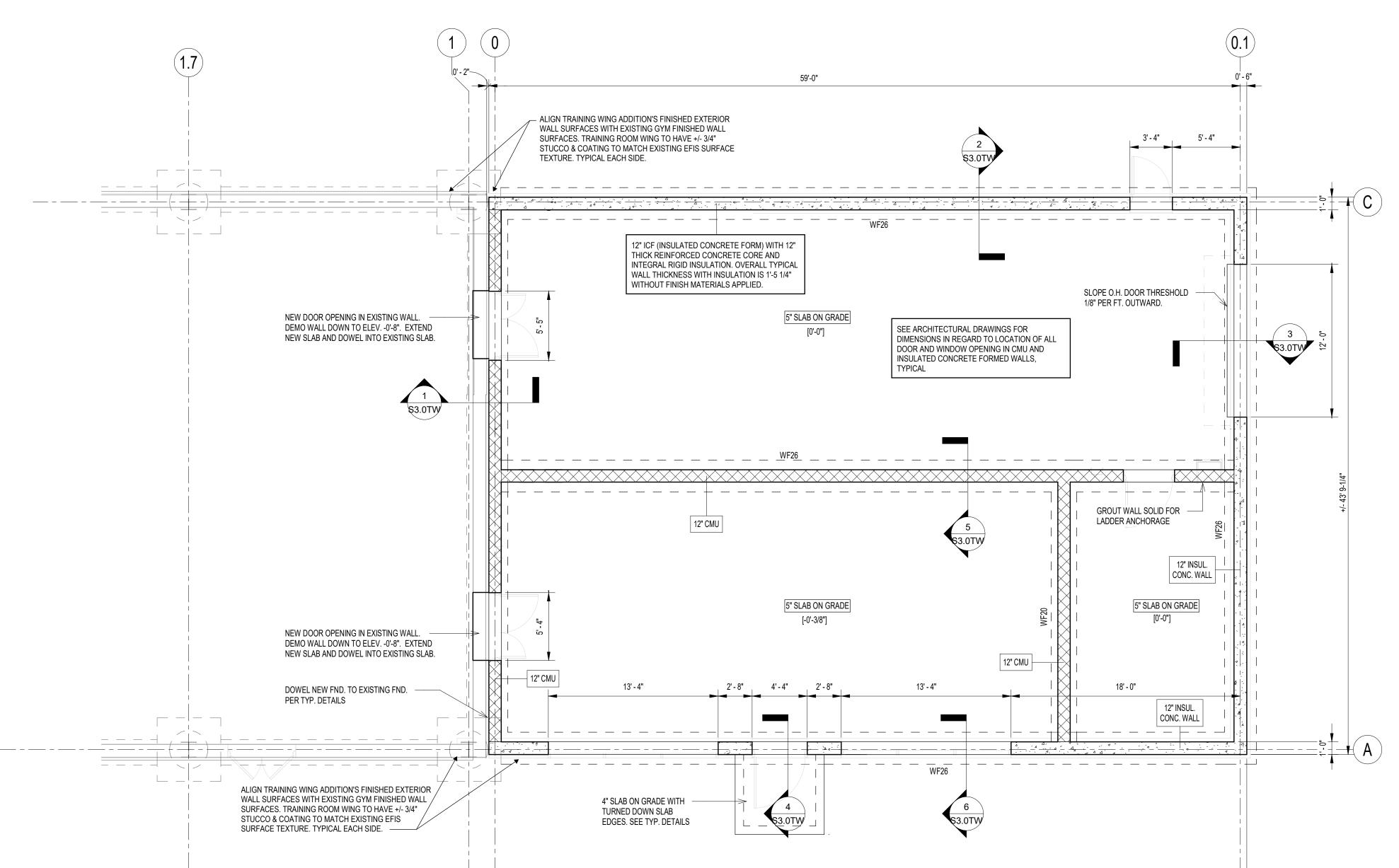


WALL LINTEL SCHEDULE					
LINTEL TAG	WALL THICKNESS	DIMENSIONS	REINFORCING STEEL		
ML1	SEE PLAN	16" DEEP CMU BOND BEAM GROUTED SOLID	2-#5 BOTTOM		











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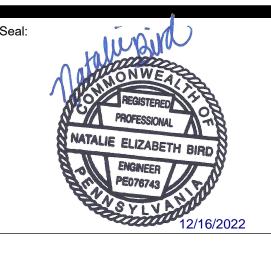
Consultants: STRUCTURAL: WZG Structural Consulting

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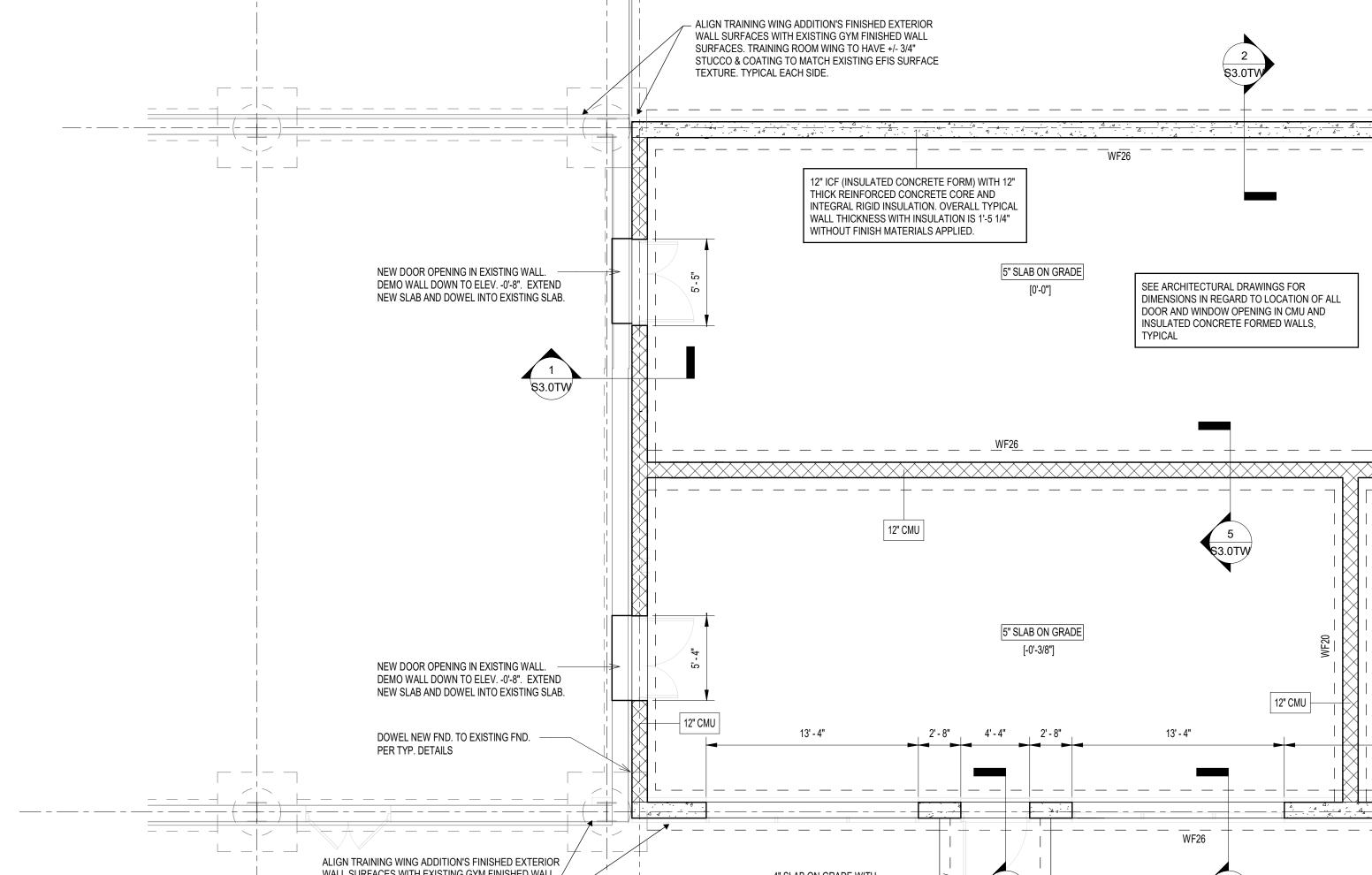
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TRAINING WING ADDITION

Prawing Title:
FOUNDATION AND MEZZANINE **PLANS** 

Issue Date: 2022-12-16 Project Number:

0336.01 Drawing Number:



1 FOUNDATION PLAN
3/16" = 1'-0"

- DATUM ELEVATION 0'-0" = 475.31' SEE S0.1 SERIES FOR GENERAL NOTES AND S2.0 SERIES FOR TYPICAL DETAILS. [-X'-XX"] INDICATES TOP OF FOOTING ELEVATION. TOP OF ALL INTERIOR FOOTING ELEV. = -0'-8" U.N.O. TOP OF ALL EXTERIOR FOOTING ELEV. = -2'-6" U.N.O. BOTTOMS OF ALL PERIMETER FOOTINGS SHALL BE 3'-0" MIN. BELOW GRADE.
- SLAB ON GRADE SHALL BE REINFORCED PER THE GENERAL NOTES. SEE GEOTECHNICAL REPORT FOR FOOTING AND SLAB SUBGRADE REQUIREMENTS. COORDINATE ALL SLAB ON GRADE RECESSES, CURBS, AND LEDGES WITH THE REQUIREMENTS OF THE ARCH. DWGS.
- 7. VERIFY SLAB DIMENSIONS WITH ARCH. DWGS. REFER TO ARCH'L DRAWINGS FOR SLAB EDGE 8. FOUNDATIONS, COLUMNS, AND PIERS ARE CENTERED ON THE GRID LINES, U.N.O.; ALL BEAMS TO BE EQUALLY SPACED BETWEEN COLUMN LINES, U.N.O.

		WALL FOOTING SCHEDULE						
	MARK	DIMENSIONS		CONTINUOUS	TRANSVERSE			
		WIDTH	DEPTH	REINF.	REINF.			

WF26 2'-6" 1'-0" 4-#5 #4 AT 48"



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Consultants:

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FRAMING PLAN NOTES: VERIFY DIMS. WITH ARCH'L DWGS. SEE ARCH'L DWGS. FOR STEEL THAT REQUIRES FIREPROOFING. ALL EXISTING DIMENSIONS SHALL BE FIELD VERIFIED.
REFER TO MECHANICAL, ELECTRICAL, AND PLUMBING DRAWINGS TO COORDINATE OPENINGS. 5. "ML" INDICATES MASONRY LINTEL. SEE SCHEDULE FOR REINF.

DENOTES SPAN OF 2" DOVETAIL METAL

ROOF DECK. SEE GENERAL NOTES FOR

ADDITIONAL DETAILS.

22' - 0" 5'-7" 5'-7" 5'-7" 5'-7" 5'-7" 5'-7" 0'-6" W16X50 (E)LOW /--HSS4X4X1/4 ─HSS4X4X1/4 /--HSS4X4X1/4 W16X40 (LOW) C8X11.5 <u>W</u>16<u>X4</u>0 \_\_W16X40\_\_ 15 \$3.0TW \_\_(E)LOW \_\_\_ ┢*╶╸╸╸*╏╸*╸╸╸*╏╴*╸╸╸╸╏╶╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸╸*╅┾╸╌╶╴╌╂╶╌╌╶╫┼║ ╶╸╸╸┪╸╸╸╸┫╸╸╸╸┃╸╸╸╸┠╸╸╸╸╅╸╸╸╽╅╸╸╸╸┃╸╸╸╸┃╸╸╸╸┃╸╸╸╸╸┩┼╜ C12X20.7 (E)LOW 6' - 6"

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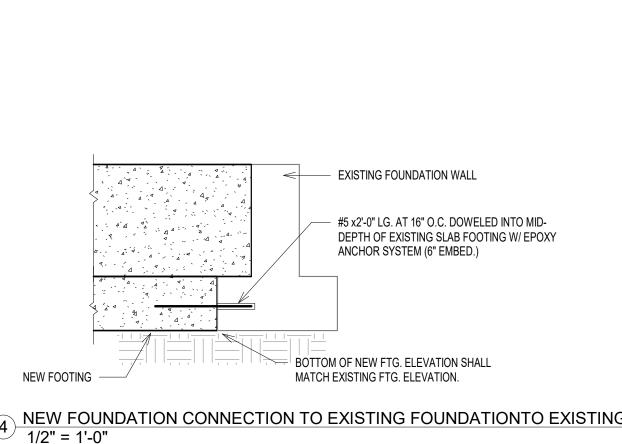
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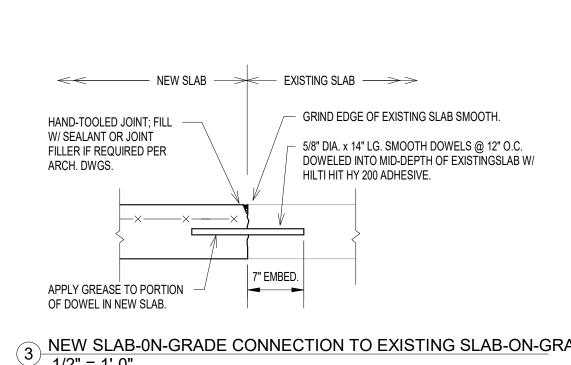
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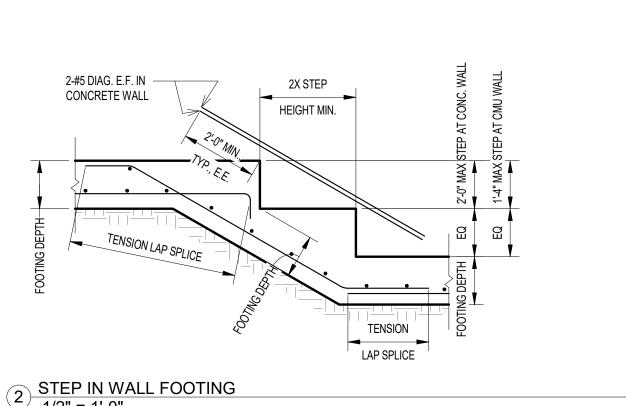
PLAN

2022-12-16

Drawing Number:







BOND BEAM REINF. —

**←** MIN.

**→** 

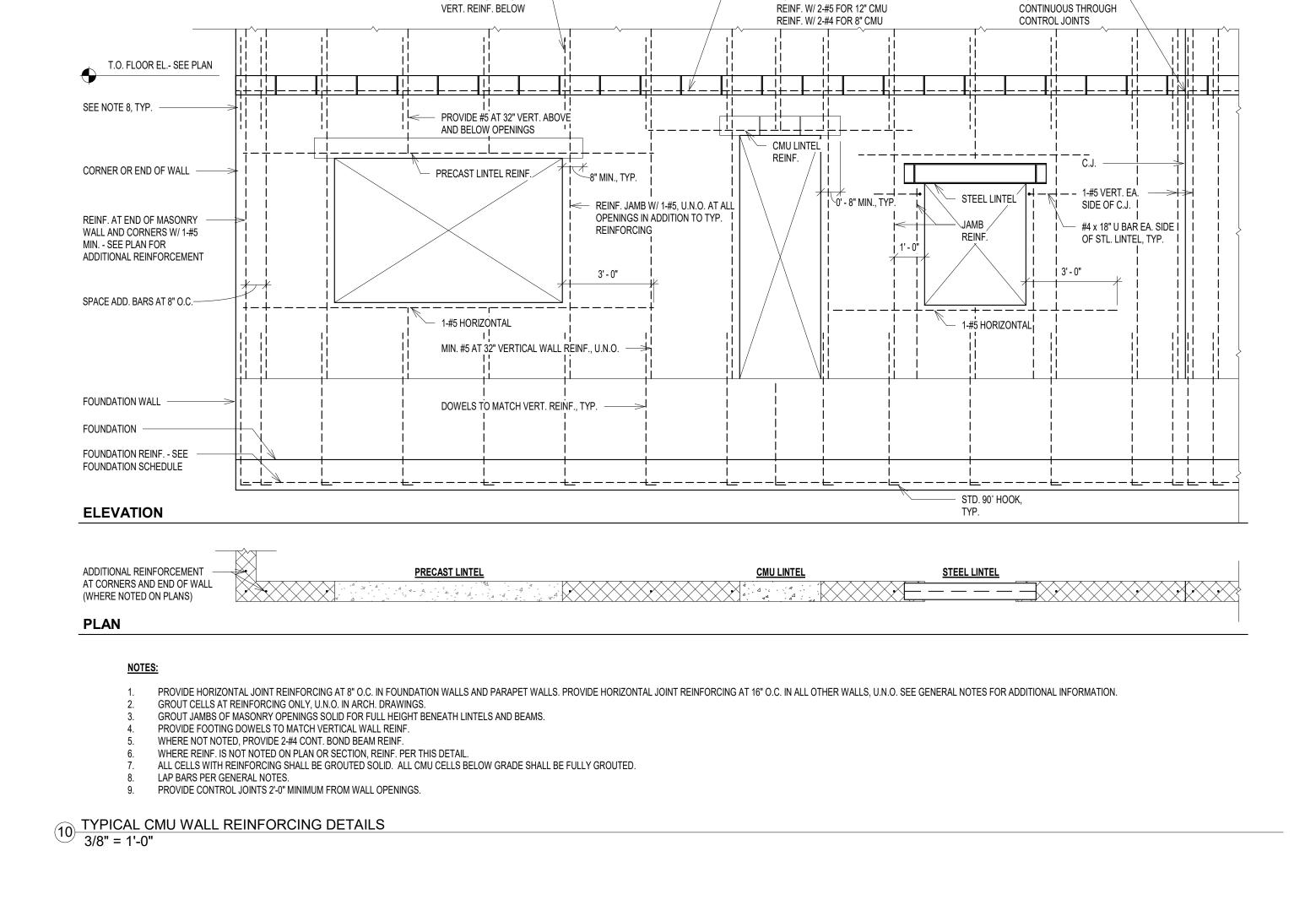
MULTIPLE SLEEVES > 8" DIA.

- #4X4'-0" DIAG, EF

- 1-#5 HEF ADDITIONAL

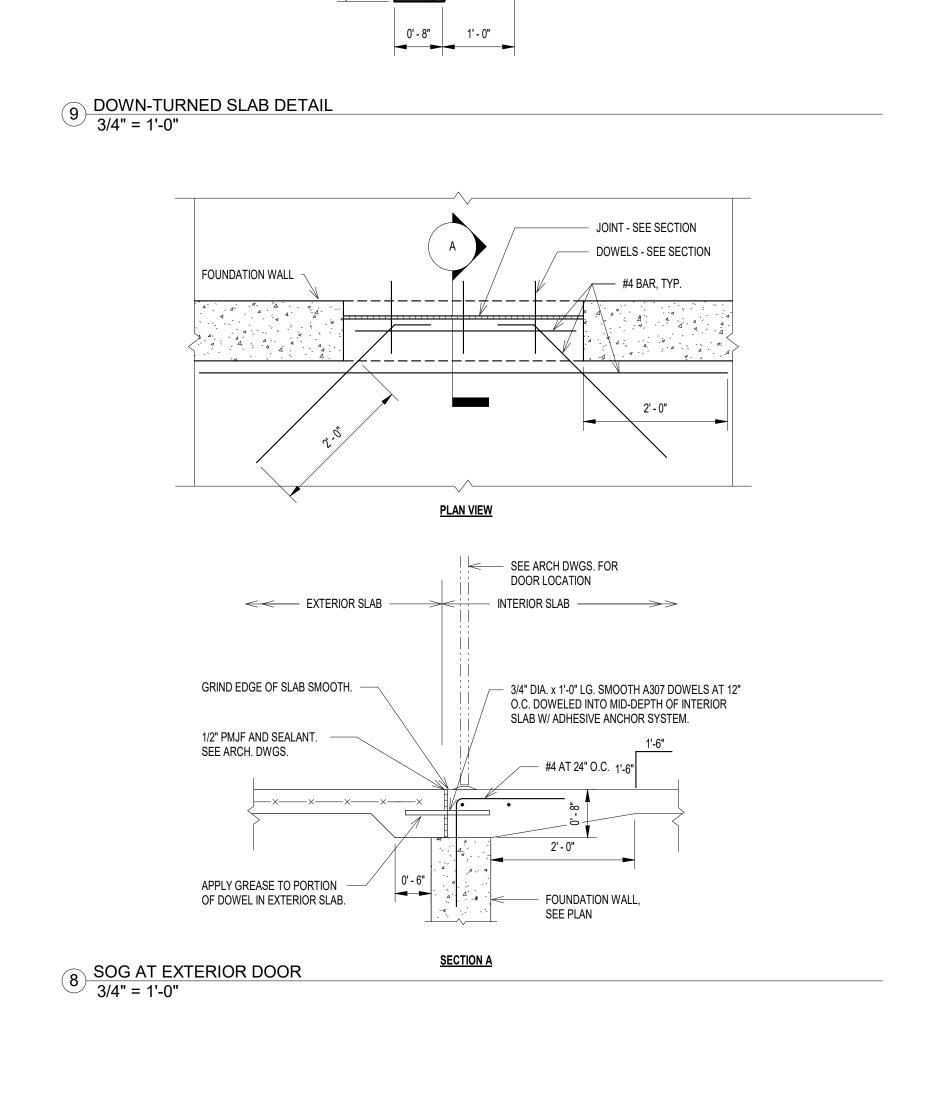
ABOVE & BELOW OPNG

(TYP 4 CORNERS)



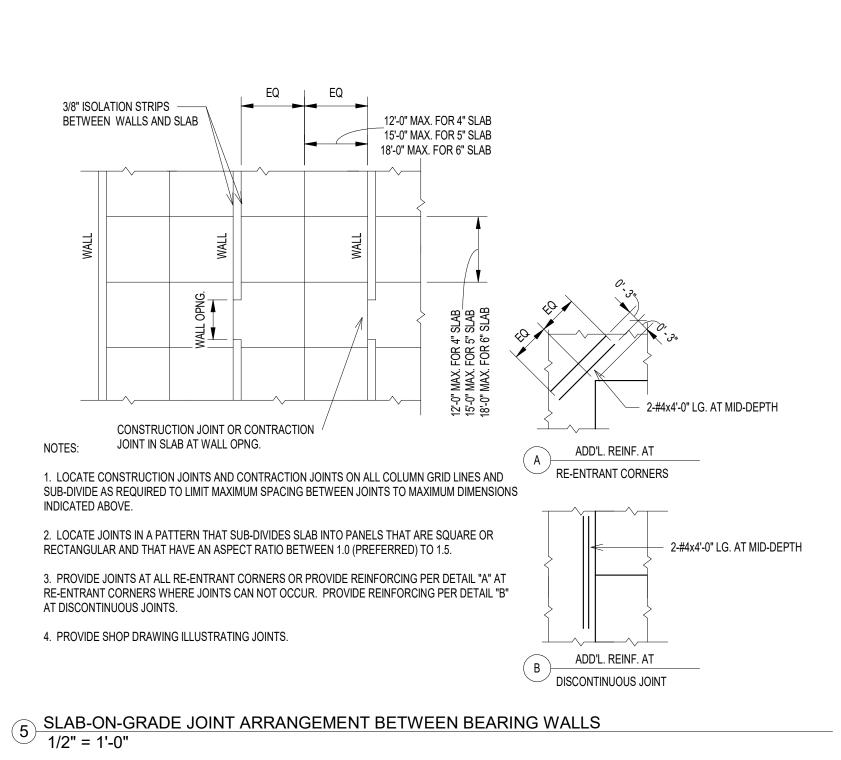
BOND BEAM:

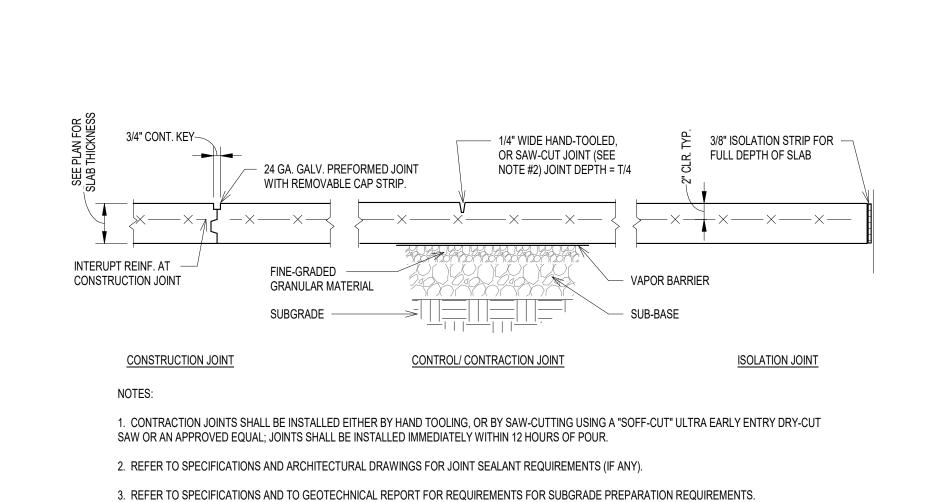
DOWELS TO MATCH



#4 NOSING -

3/4" CHAMFER





1 SLAB ON GRADE CONSTRUCTION 1" = 1'-0"



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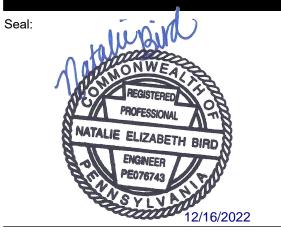
Consultants: STRUCTURAL: WZG Structural Consulting Engineers, Inc. 1137 Gravel Pike

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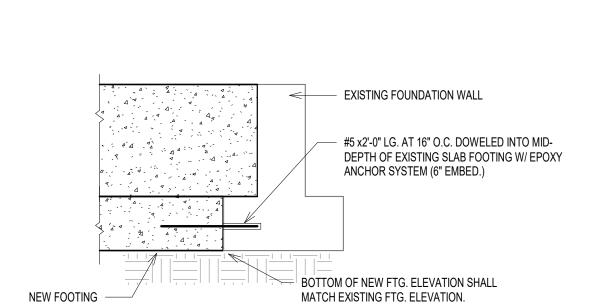
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TRAINING WING ADDITION

Drawing Title: **DETAILS** 

Issue Date: 2022-12-16 Project Number: 0336.01 Drawing Number:



INFILL BEAM POCKET SOLID — W/ MASONRY AFTER INSTALLING STEEL BEAM; PROVIDE MASONRY

ANCHORS @ 12" O.C. E.W. ON

BOTH SIDES OF BEAM WEB

PROVIDE LONG SLOTTED -

BEARING PL. 5/8"x7.5"x1'-0"

W/(2)-1/2" DIA. ANCHOR RODS

GROUT BLOCK CORES SOLID -

BEAM BEARING PARALLEL TO MASONRY WALL

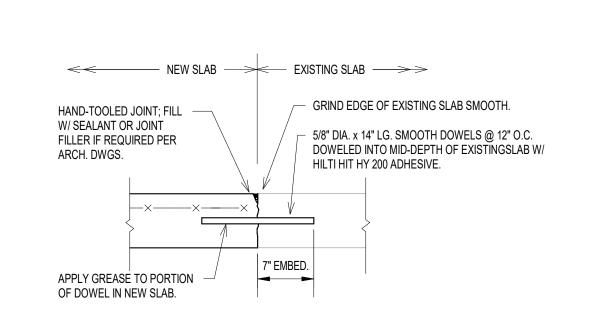
BELOW BEAM BEARING FOR

HOLES IN BEAM

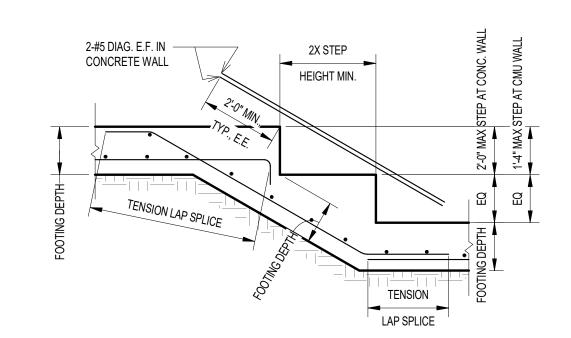
W/6" EMBED.

24" MIN. WIDTH

<sup>'/</sup> 3/4" = 1'-0"



HORIZ REINF



EFFECTIVE OPENING

MULTIPLE SLEEVES < 8" DIA.

1. OPENINGS SMALLER THEN 12"x12" REQUIRE NO ADDITIONAL REINFORCING STEEL.

3. INDIVIDUAL PIPE SLEEVES < 12" DIAMETER REQUIRE NO ADDITIONAL REINFORCING.

INSTALL ADD'L VERT BARS ON -

EACH SIDE OF OPENING EQUAL

TO ONE HALF OF THE VERT REINF

INTERRUPTED BY THE OPENING.

UNINTERRUPTED BETWEEN SLEEVES.

6 OPENING IN CONCRETE WALL 1/2" = 1'-0"

NOTES:

3'-0" MAX.

PERMITTED, U.N.O.

2. RECTANGULAR OPENING SHOWN; ADDITIONAL REINFORCING STEEL AROUND INDIVIDUAL ROUND OPENINGS > 12" DIAMETER SIMILAR.

4. SPACE GROUPED SLEEVES PER DETAILS "A" AND "B" ABOVE AND TREAT AS A SINGLE EFFECTIVE RECTANGULAR OPENING. PROVIDE ADDITIONAL

REINFORCING STEEL AROUND EFFECTIVE OPENING AS SHOWN ABOVE AND INSTALL REQUIRED HORIZONTAL AND VERTICAL REINFORCING STEEL

2' - 0" TYP., E.E.

4 NEW FOUNDATION CONNECTION TO EXISTING FOUNDATIONTO EXISTING SLAB-ON-GRADE 1/2" = 1'-0"

- 3/4" THK. 8000 PSI

#4 IN EA. GROUTED CORE BELOW BEAM BEARING

NON-SHRINK GROUT

PROVIDE TENSION LAP SPLICES BETWEEN ALL

CORNER BARS AND HORIZ WALL REINFORCING.

CORNER BARS

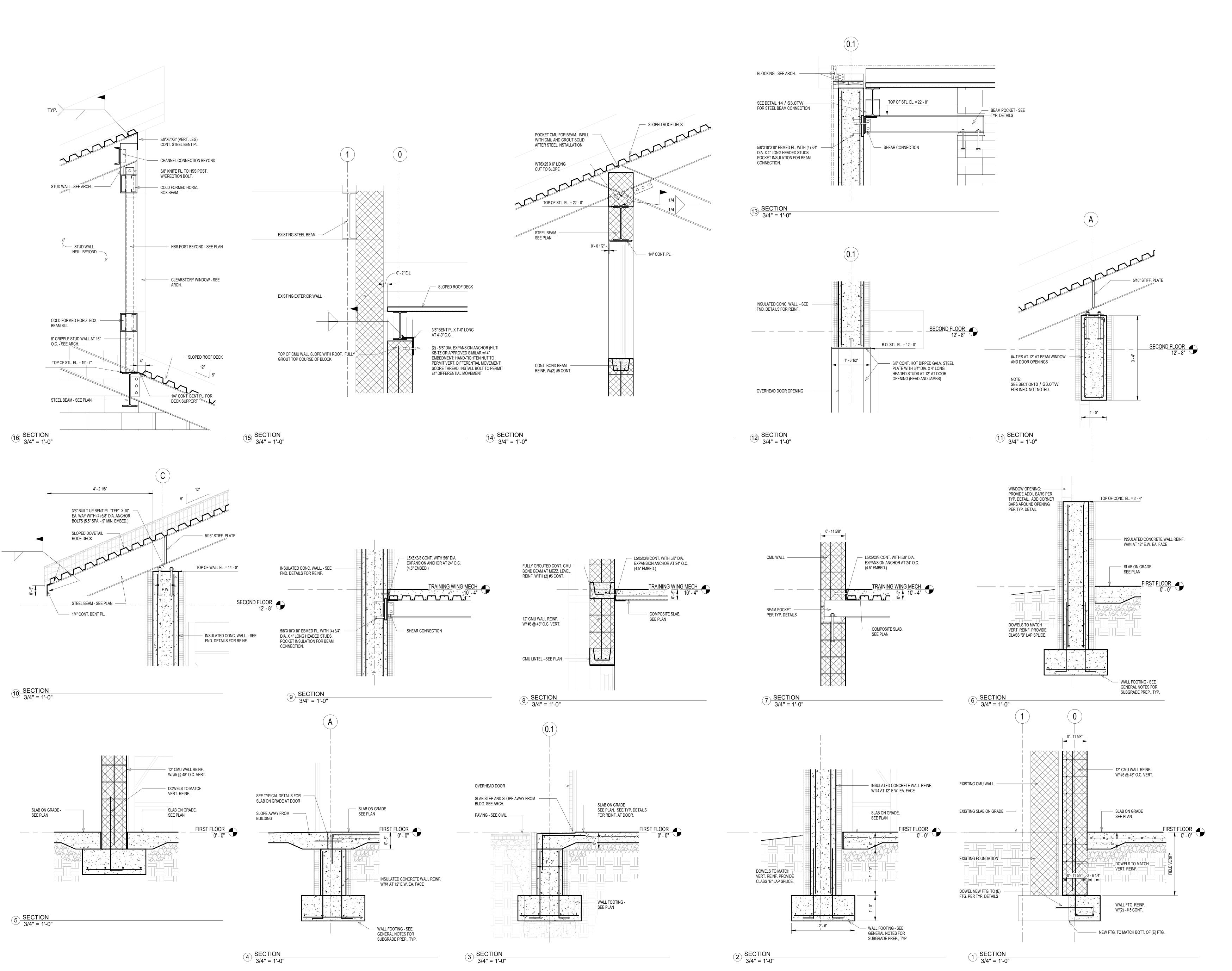
HORIZONTAL CORNER BARS SHALL MATCH SIZE AND SPACING OF HORIZONTAL REINFORCING.

HORIZ REINF.

7 CORNER BAR REINFORCING FOR CONCRETE WALLS
1/2" = 1'-0"

3 NEW SLAB-0N-GRADE CONNECTION TO EXISTING SLAB-ON-GRADE 1/2" = 1'-0"

2 STEP IN WALL FOOTING 1/2" = 1'-0"





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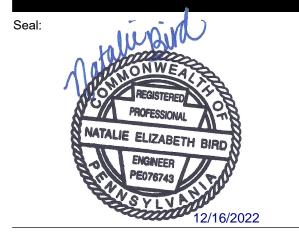
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ISSUE LOG

Distribution Date No.

ISSUE FOR PERMIT 12/16/22

Client:
PA GAME COMMISSION

TRAINING WING ADDITION

Drawing Title:
SECTIONS

Issue Date:
2022-12-16
Project Number: D
0336.01

Drawing Number:

S3.0TW